INTERIOR GIRDER MOMENT TABLE						
		0.4 Span 1 or 0.6 Span 3	Pier 1 or Pier 2	0.5 Span 2		
Is	(in 4)	1,480	1,480	1 <b>,</b> 480		
Ic (n)	(in 4)	4,974		4,974		
Ic(3n)	(in4)	3,699		3,699		
Ss	(in 3)	140	140	140		
Sc (n)	(in 3)	232		232		
Sc(3n)	(in³)	208		208		
Z	(in³)	160	160	160		
DC1	(K/ft.)	0.644	0.644	0.644		
M DC1.	('K)	54.2	102.1	<i>53.8</i>		
DC2	(K/ft.)	0.129	0.129	0.129		
M DC2	('K)	13.0	15.0	16.3		
DW	(K/ft.)	0.279	0.279	0.279		
Mow	('K)	28.1	<i>32.3</i>	<i>35.2</i>		
Mt + IM	('K)	292.9	174.1	344.2		
My (Strength I) ('K)		638.8	499.5	742.7		
øf Mn, øf Mnc,('K)		1301.6		1301.6		
fs DC1	(k.s.i.)	4.65	8.75	4.61		
fs DC2	(k.s.i.)	0.75	1.29	0.94		
fs DW	(k.s.i.)	1.62	2.77	2.03		
fs 1.3(4+IM)	(k.s.i.)	19.70	19.40	23.14		
fs (Service II)	(k.s.i.)	26.72	32.21	30.72		
fs (Total)(Strength1)		<i>35.69</i>	42.82	41.14		
Vf	(K)	25.0		23.7		

 $I_{S}$ ,  $S_{S}$ : Non-composite moment of inertia and section modulus of the steel section used for computing  $f_{S}$  (Toral-Strength I, and Service II) due to non-composite dead loads (in.<sup>4</sup> and in.<sup>3</sup>).

Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing  $f_S$  (Total-Strength I, and Service II) due to short-term composite live loads (in.<sup>4</sup> and in.<sup>3</sup>).

 $I_{\mathcal{C}}(3n),~S_{\mathcal{C}}(3n).$  Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing  $f_{\mathcal{S}}$  (Total-Strength I, and Service II) due to long-term

composite (superimposed) dead loads (in.4 and in.3).

Z: Plastic Section Modulus of the steel section in non-composite areas. Omit line in Moment Table if not used in design calculations (in.3).

DC1: Un-factored non-composite dead load (kips/ft.).

Mpc1: Un-factored moment due to non-composite dead load (kip-ft.).

DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).

Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).  $M_{DC2}$ :

DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.). Un-factored mament due to long-term composite (superimposed

future wearing surface only) dead load (kip-ft.).

Mit Imp: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).

Mu (Strength I): Factored design moment (kip-ft.).

 $1.25 (M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M_{4} + Imp$ 

 $\phi_f$   $M_n$ : Compact composite positive moment capacity computed according to Article 6.10.7.1 (kip-ft.).

 $\phi_f$   $M_{nc}$ : Compact non-composite negative moment capacity computed

according to Article A6.1.1 (kip-ft.).

 $f_S$  (Service II): Sum of stresses as computed from the moments below (ksi).  $M_{DCI} + M_{DCQ} + M_{DW} + 1.3 M_{\odot} + Imp$   $f_S$  (Total)(Strength I): Sum of stresses as computed from the moments below on

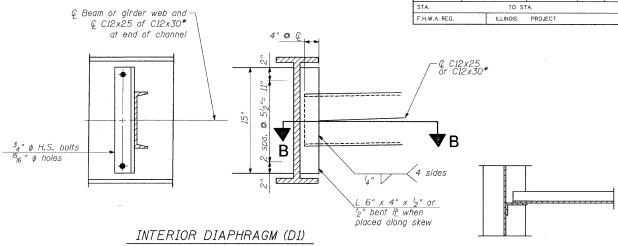
non-compact section (ksi),

1.25 (Mpc) +  $M_{DC2}$ ) + 1.5  $M_{DW}$  + 1.75  $M_{L}$  + Imp  $V_f$ : Factored shear range computed according to Article 6.10.10.

# CONTRACT NUMBER 91402

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.			
N. LINCOLN AVE.	07-25932-00-BR	CHAMPAIGN	50	23			
STA. TO STA.							

SECTION B-B



INTERIOR GIRDER REACTION TABLE				
		Abuts.	Pier 1 or Pier 2	
R DC1	(K)	8.4	28.4	
R DC2	(K)	1.8	5 <b>.</b> 5	
R DW	(K)	4.0	11.9	
R 4 + IM	(K)	45.9	65.4	
R (Total)	(K)	60.1	111.2	

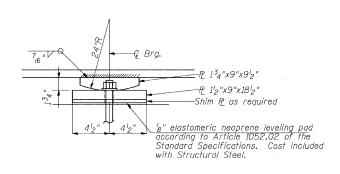
#### Notes:

(30 Required)

Two hardened washers required for each set of oversized

\*Alternate channels are permitted to facilitate material acquisition. Calculated weight of structural steel is based on the lighter section.

All cross frames or diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual cross frames or diaphragms at supports may be temporarily disconnected to install bearing anchor rods.



## ELEVATION AT PIER

# 1<sup>3</sup>8" ¢ Holes-1" deep in top ₽ for 14" opintles. Thread of press 1'4" Ø PINTLE 1" \ x 14" Anchor Bolts (ASTM F1554, Grade 36) with $2^{l}_{4}$ "x $2^{l}_{4}$ "x $^{l}_{6}$ " P washer under nut $1^{l}_{2}$ " $\phi$ Holes in bottom P. 18/2" SECTION B-B

### FIXED BEARING

### Notes:

- 1. Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. ASTM A307 Grade C anchor bolts may be used in lieu of ASTM F1554 Grade 36 (Fy=36ksi). The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.
- 2. Anchor bolts at fixed bearings may be either cast in place or installed in holes drilled after the supported
- 3. Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.
- 4. Two  ${}^{\prime}{}_{8}{}^{\prime\prime}$  adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details.
- 5. The structural steel plates of the bearing assembly shall conform to the requirements of AASHTO M270, Grade 50W.

FRAMING PLAN & BEAM DETAILS
SECTION: 07-25932-00-BR
CHAMPAIGN COUNTY
Q STATION 10+00

NORTH LINCOLN AVE.

SECTION 07-25932-00-BR

CHAMPAIGN COUNTY

SHEET 23 OF 50 SHEETS

06156